

vigas hiperestáticas

ESTRUCTURAS 2

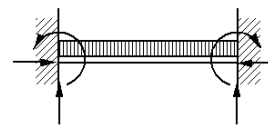
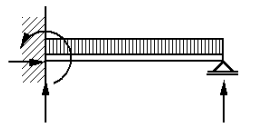
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profesora: Verónica Veas

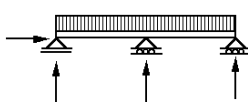
ayudante: Preeti Bellani

Vigas Hiperestáticas

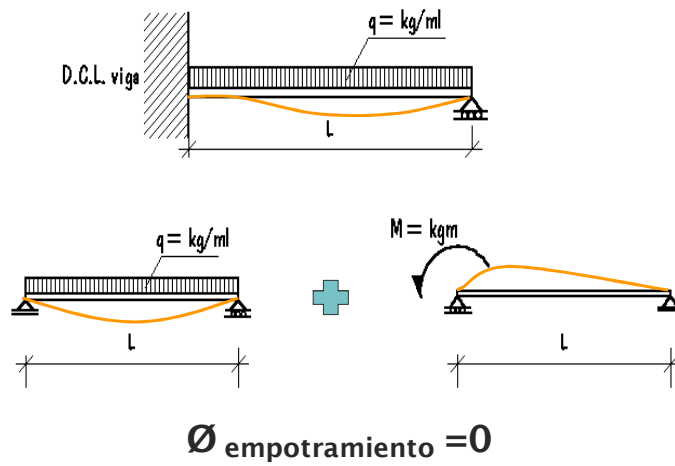
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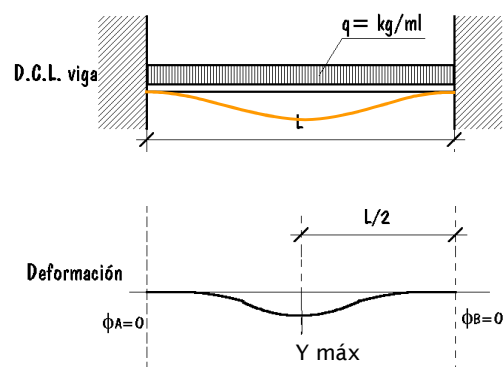
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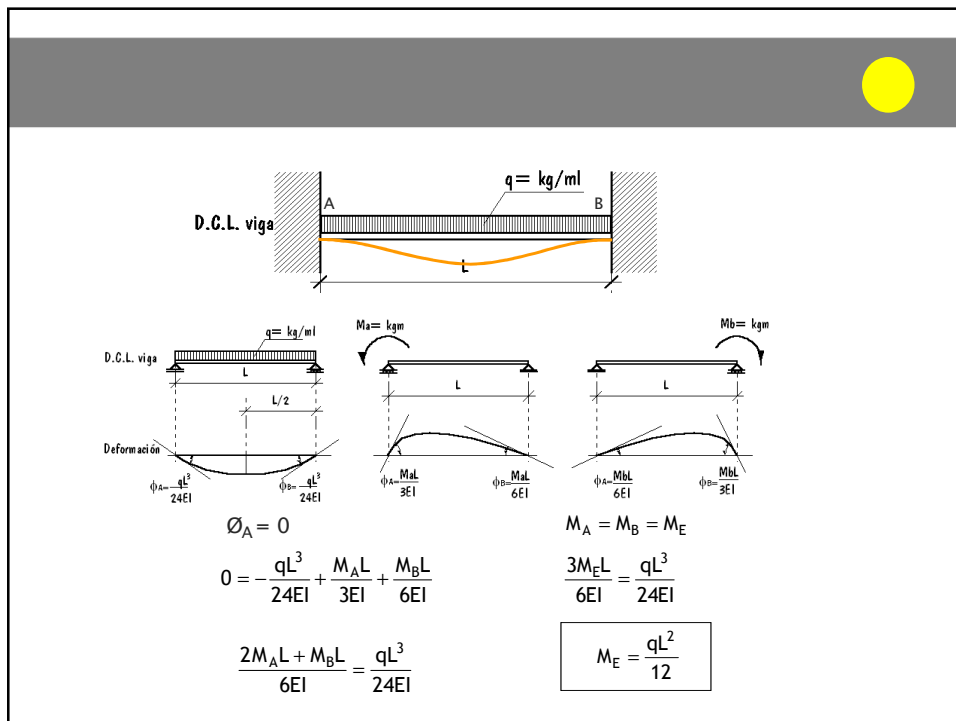
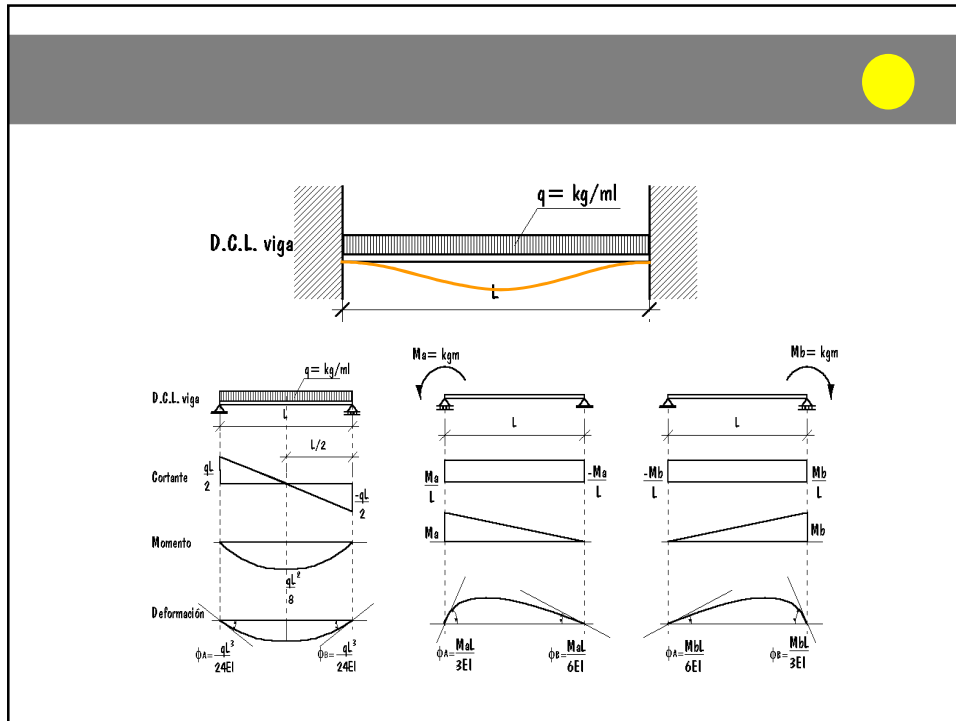


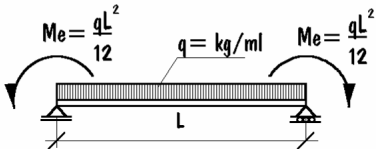
Concepto de vigas hiperestáticas por empotramiento



Ejemplo Viga bi-empotrada con carga uniformemente repartida







$M_e = \frac{qL^2}{12}$ $q = kg/ml$ $M_e = \frac{qL^2}{12}$

$R_a = R_b = \frac{qL}{2}$

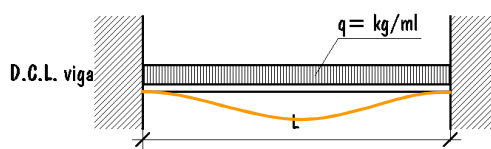
El momento máximo en una viga simétrica se encuentra en $X=L/2$

$M_{(L/2)} = \frac{qL}{2} \frac{L}{2} - \frac{q}{2} \left(\frac{L}{2}\right)^2 - M_e$

$M_{(L/2)} = \frac{qL^2}{4} - \frac{qL^2}{8} - \frac{qL^2}{12}$

$M_x = \frac{qLx}{2} - \frac{qx^2}{2} - M_e$

$M_{MAX} = \frac{qL^2}{24}$



D.C.L. viga

$q = kg/ml$

$Y_{L/2} = \frac{5qL^4}{384EI}$

$Y_{L/2} = \frac{ML^2}{16EI}$

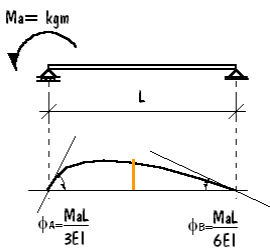
$Y_{L/2} = \frac{ML^2}{16EI}$

Deformación: $\phi_a = -\frac{qL^3}{24EI}$, $\phi_b = -\frac{qL^3}{24EI}$

$M_a = kqm$, $M_b = kqm$

$\phi_a = \frac{ML}{3EI}$, $\phi_b = \frac{ML}{6EI}$

$\phi_a = \frac{ML}{6EI}$, $\phi_b = \frac{ML}{3EI}$



$M_a = kgm$

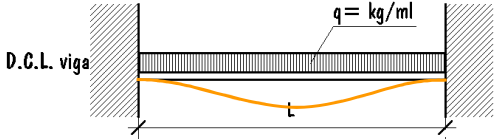
L

$\phi_A = \frac{MaL}{3EI}$

$\phi_B = \frac{MaL}{6EI}$

$$Y_{L/2} = \frac{ML^2}{16EI}$$

$$Y_{L/2} = \frac{qL^2}{12} \frac{L^2}{16EI}$$

$$Y_{L/2} = \frac{qL^4}{192EI}$$


D.C.L. viga

$q = kg/ml$

L

D.C.L. viga

$q = kg/ml$

L

$L/2$

Deformacion

$\phi_A = \frac{qL^3}{24EI}$

$\phi_B = \frac{qL^3}{24EI}$

$M_a = kgm$

L

$\phi_A = \frac{MaL}{3EI}$

$\phi_B = \frac{MaL}{6EI}$

$M_b = kgm$

L

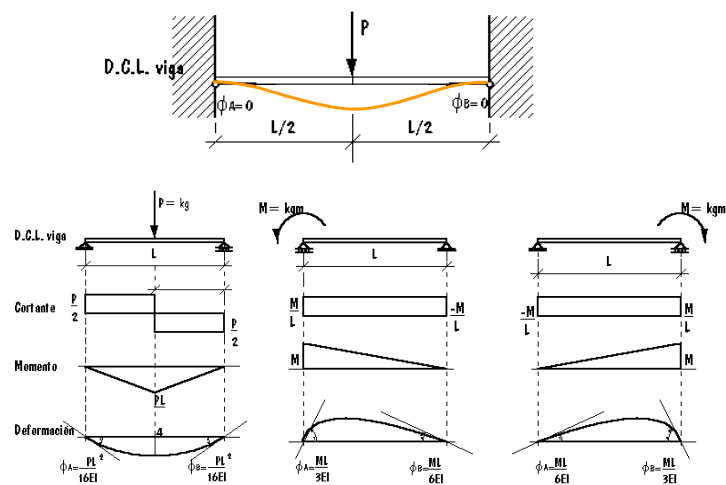
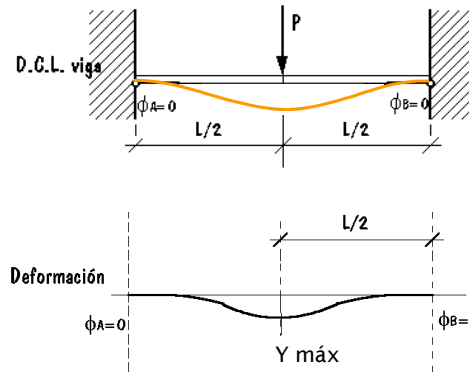
$\phi_A = \frac{MbL}{6EI}$

$\phi_B = \frac{MbL}{3EI}$

$$Y_{MAX} = \frac{5qL^4}{384EI} - \frac{qL^4}{192EI} - \frac{qL^4}{192EI}$$

$$Y_{MAX} = \frac{qL^4}{384EI}$$

Ejemplo Viga bi-empotrada con carga puntual al centro



D.C.L. viga

Deformación

$\phi_A = \frac{PL^2}{16EI}$ $\phi_B = \frac{PL^2}{16EI}$

$\phi_A = \frac{ML}{3EI}$ $\phi_B = \frac{ML}{6EI}$

$\phi_A = \frac{ML}{6EI}$ $\phi_B = \frac{ML}{3EI}$

$\phi_A = 0$

$M_A = M_B = M_E$

$$0 = \frac{-PL^2}{16EI} + \frac{M_A L}{3EI} + \frac{M_B L}{6EI}$$

$$\frac{2M_A L + M_B L}{6EI} = \frac{PL^2}{16EI}$$

$$\frac{3M_E L}{6EI} = \frac{PL^2}{16EI}$$

$$M_E = \frac{PL^2}{8}$$

$M_A = \frac{PL}{8}$ $M_B = \frac{PL}{8}$

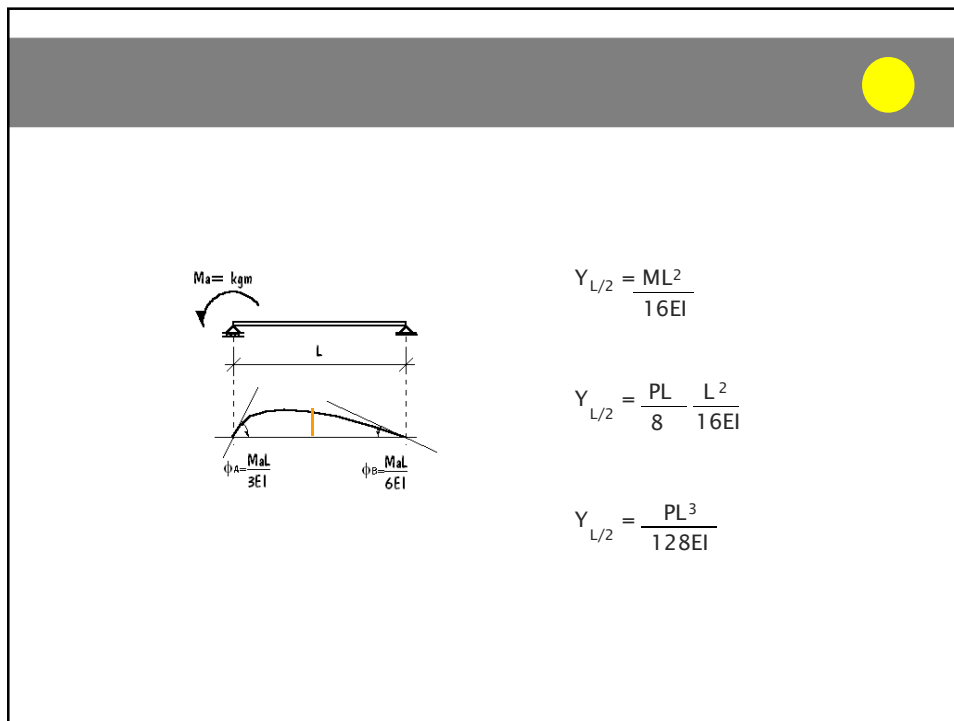
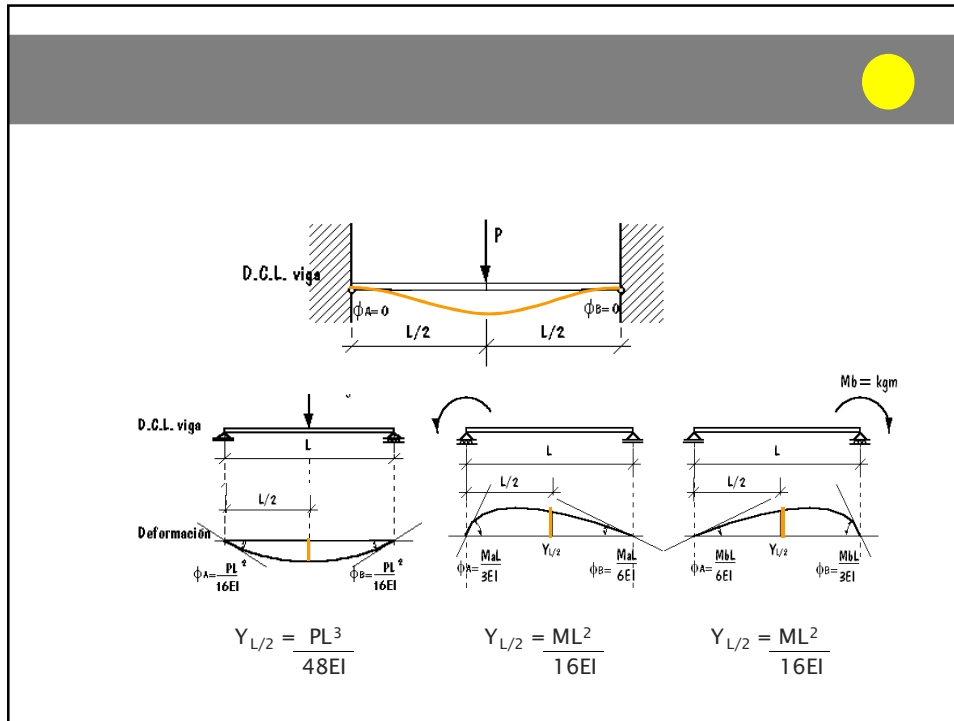
$R_A = R_B = \frac{P}{2}$

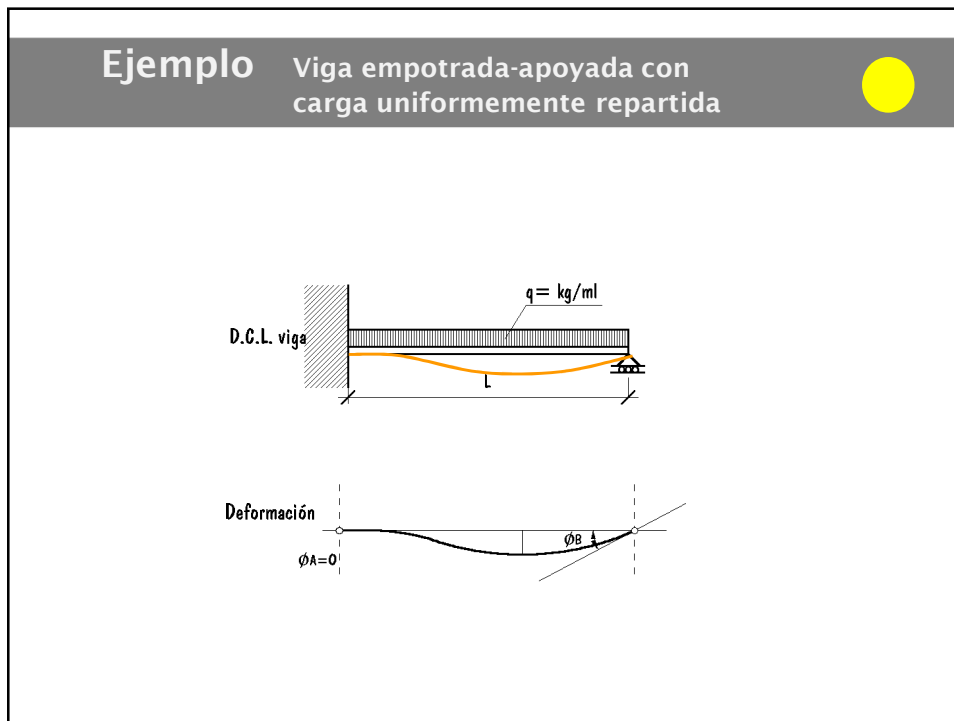
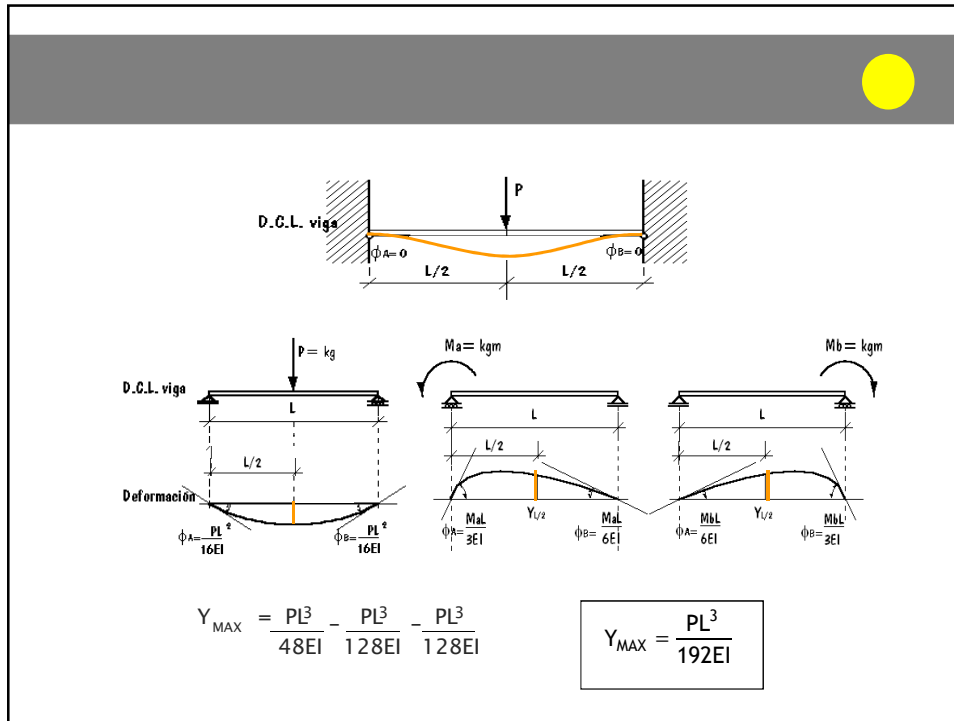
El momento máximo en una viga simétrica se encuentra en $X=L/2$

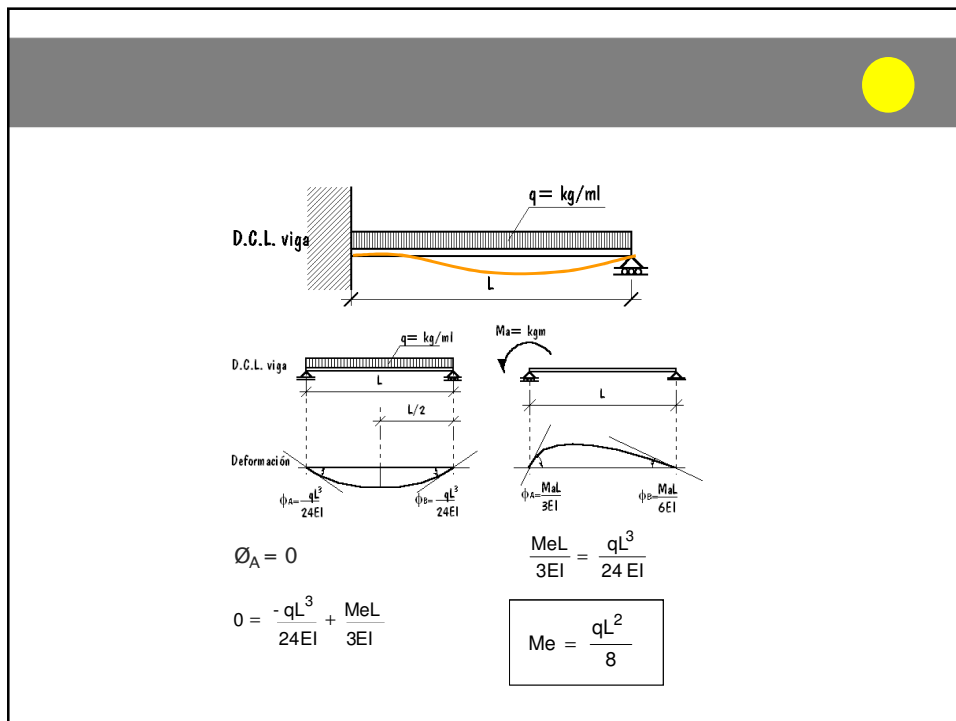
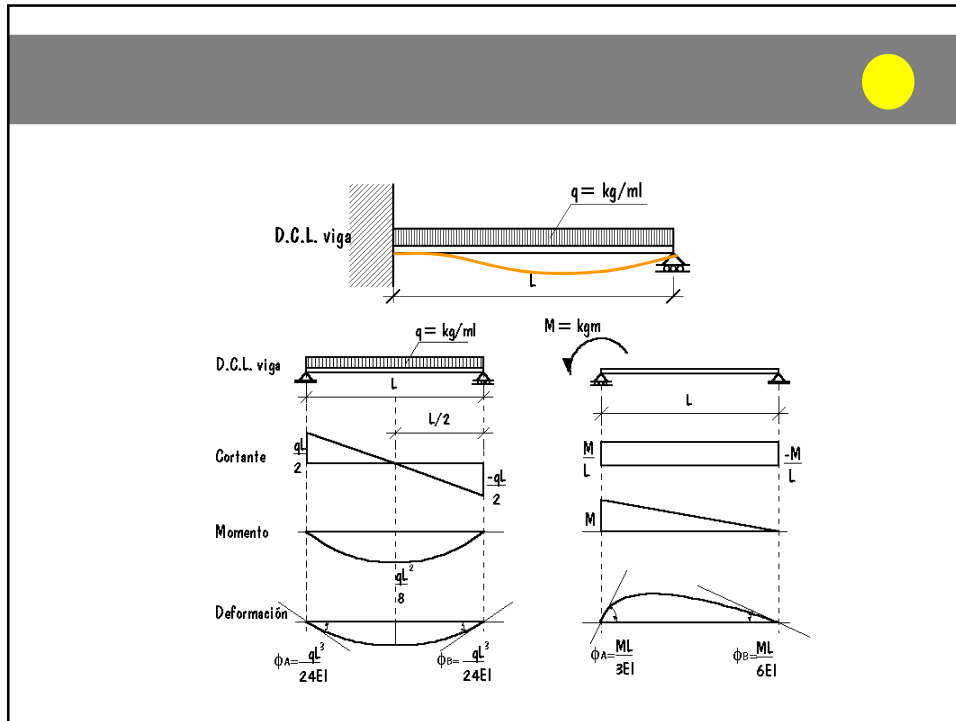
$$M_{(L/2)} = -\frac{PL}{8} + \frac{P}{2} \cdot \frac{L}{2}$$

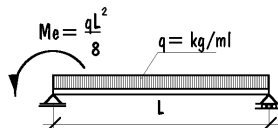
$$M_{MAX} = \frac{PL}{8}$$

$$M_x = \frac{-PL}{8} + \frac{PX}{2}$$



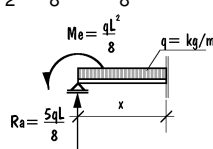






$R_a = \frac{qL}{2} + \frac{M_e}{L} = \frac{qL}{2} + \frac{qL}{8} = \frac{5qL}{8}$
 $M_x = \frac{5qLx}{8} - \frac{qx^2}{2} - M_e$

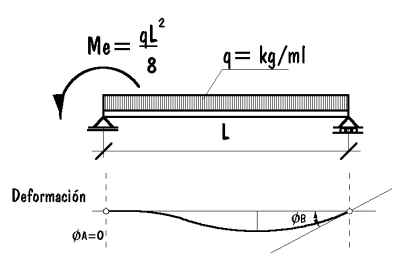
$R_b = \frac{qL}{2} - \frac{M_e}{L} = \frac{qL}{2} - \frac{qL}{8} = \frac{3qL}{8}$
 $M_{MAX} = \frac{25qL^2}{64} - \frac{25qL^2}{128} - \frac{qL^2}{8}$



$V_x = 0$
 $R_a = \frac{5qL}{8}$

$M_{MAX} = \frac{9qL^2}{128}$

$\frac{5qL}{8} - qx = 0 \quad x = \frac{5L}{8}$




$EI \frac{d^2y}{dx^2} = \frac{5qLx}{8} - \frac{qL^2}{8} - \frac{qx^2}{2}$

$EI \frac{dy}{dx} = \frac{5qLx^2}{16} - \frac{qL^2x}{8} - \frac{qx^3}{6} + C_1$

$EI y = \frac{5qLx^3}{48} - \frac{qL^2x^2}{16} - \frac{qx^4}{24} + C_1x + C_2$

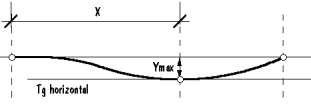
Condiciones de apoyo

Si $x=0$ $C_1=0$
 Si $x=0$ ó $x=L$ $C_2=0$



Flecha máx en $dy/dx=0$

Deformación




$$\frac{5qLx^2}{16} - \frac{qL^2x}{8} - \frac{qx^3}{6} = 0$$

$$x = 0,58L$$

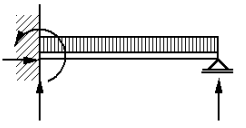
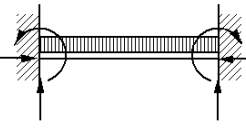
$$Y = \frac{5qL}{48EI} (0,58L)^3 - \frac{qL^2}{16EI} (0,58L)^2 - \frac{q}{24EI} (0,58L)^4$$

$$Y_{MAX} = \frac{qL^4}{185EI} = 0,005 \frac{qL^4}{EI}$$

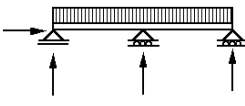
Vigas Hiperestáticas



a

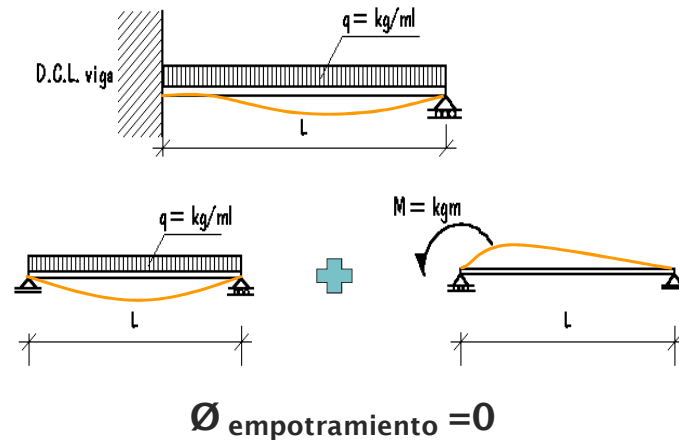



b



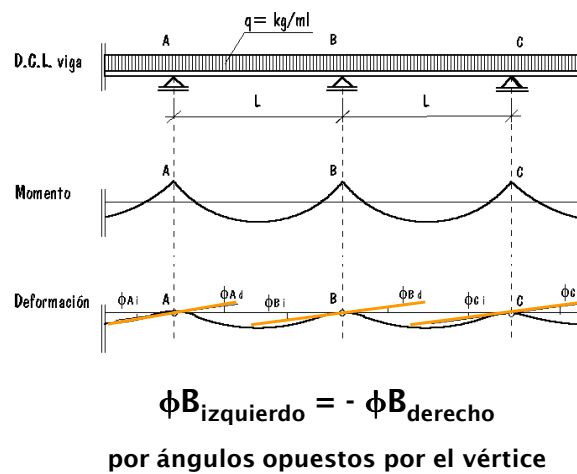
Concepto de vigas hiperestáticas por empotramiento

a

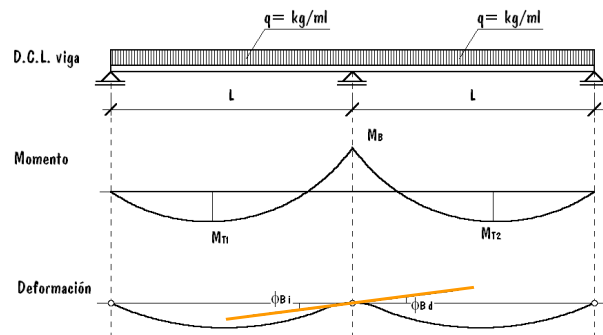


Concepto de vigas hiperestáticas por continuidad

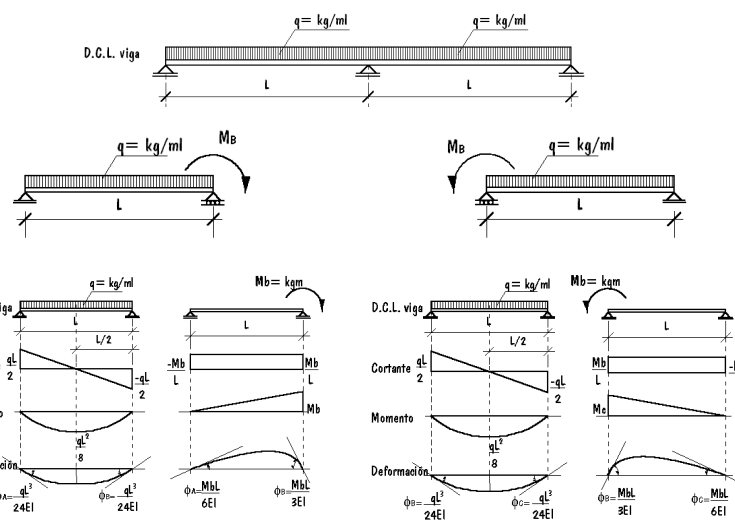
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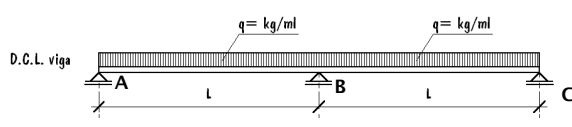
Ejemplo Viga de dos tramos con carga uniformemente repartida



$$\phi_{B_{izquierdo}} = -\phi_{B_{derecho}}$$



☀



D.C.L. viga

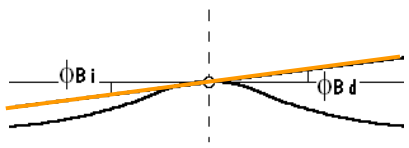
Se igualan los valores de ángulos a ambos lados del apoyo B para determinar el momento de continuidad entre ambos tramos.

$$\sum \phi_B \text{ izquierdo} = -\sum \phi_B \text{ derecho}$$

$$-\frac{qL^3}{24EI} + \frac{M_B L}{3EI} = -\left(-\frac{qL^3}{24EI} + \frac{M_B L}{3EI}\right)$$

$$\frac{2M_B L}{3EI} = \frac{qL^3}{12EI} \quad \dots \cdot EI/L$$

$$M_B = \frac{qL^2}{8EI}$$



☀

$$R_B = \frac{qL}{2} + \frac{M_B}{L} = \frac{5qL}{8}$$

$$R_C = \frac{qL}{2} - \frac{M_B}{L} = \frac{3qL}{8}$$

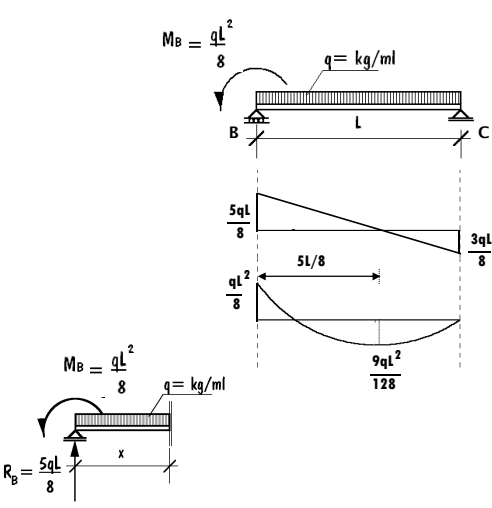
$$M_x = \frac{5qLx}{8} - \frac{qx^2}{2} - \frac{qL^2}{8}$$

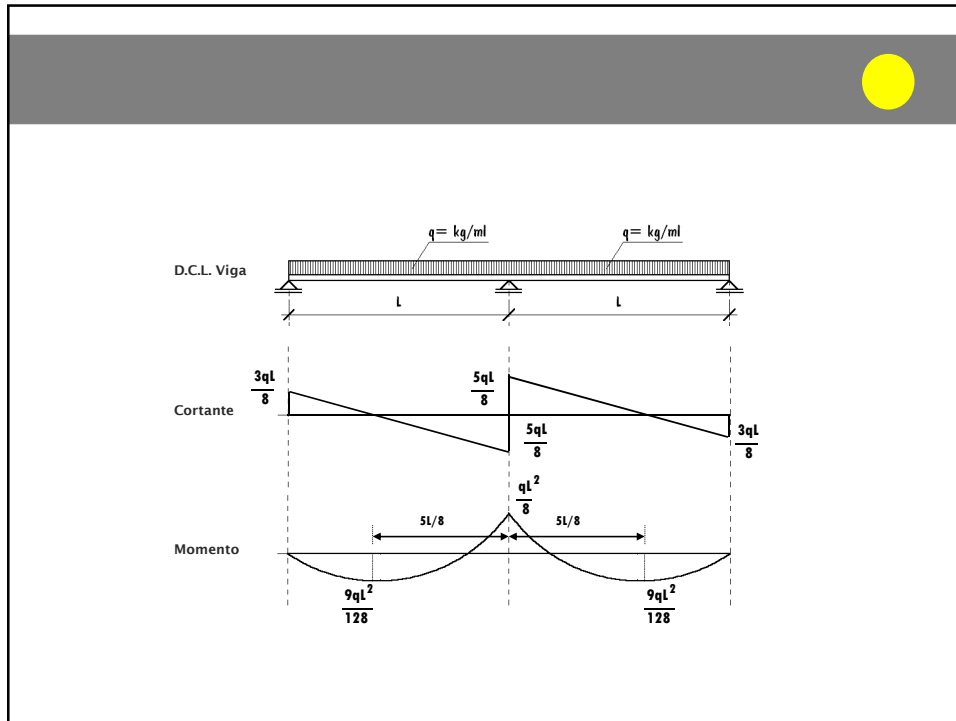
$$V_x = 0$$

$$\frac{5qL}{8} - qx = 0 \quad x = \frac{5L}{8}$$

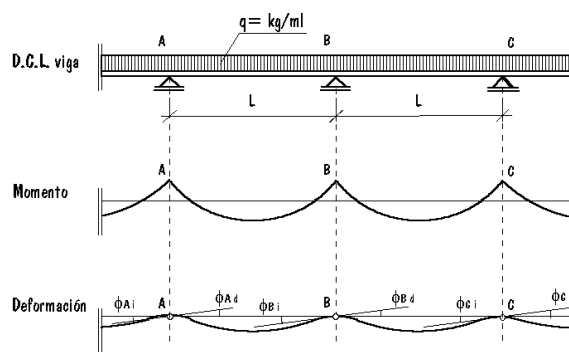
$$M_{MAX} = \frac{25qL^2}{64} - \frac{25qL^2}{128} - \frac{qL^2}{8}$$

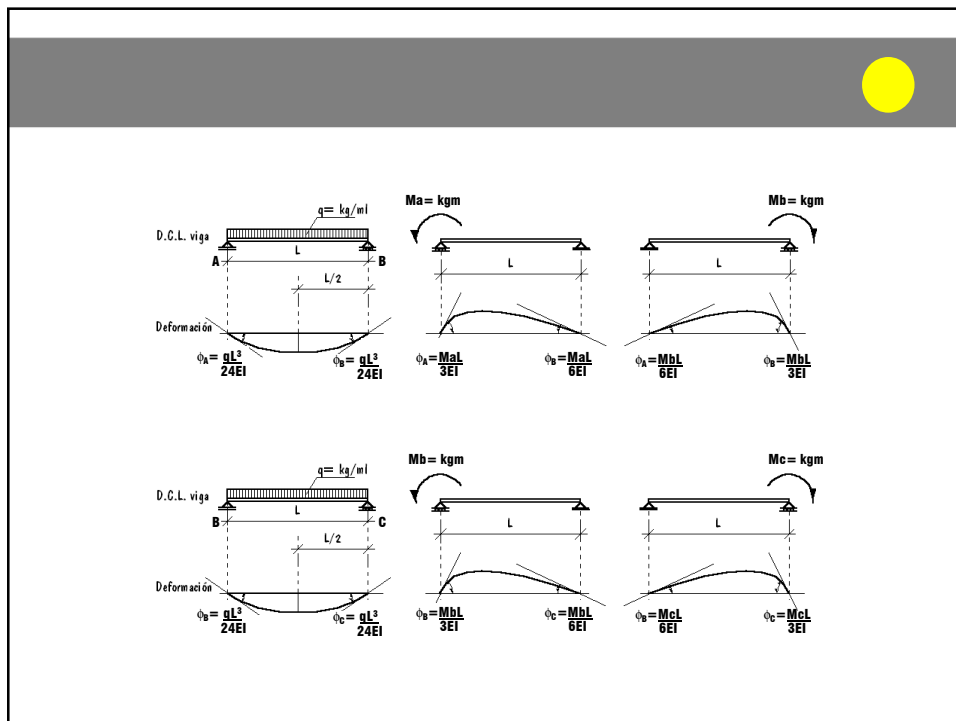
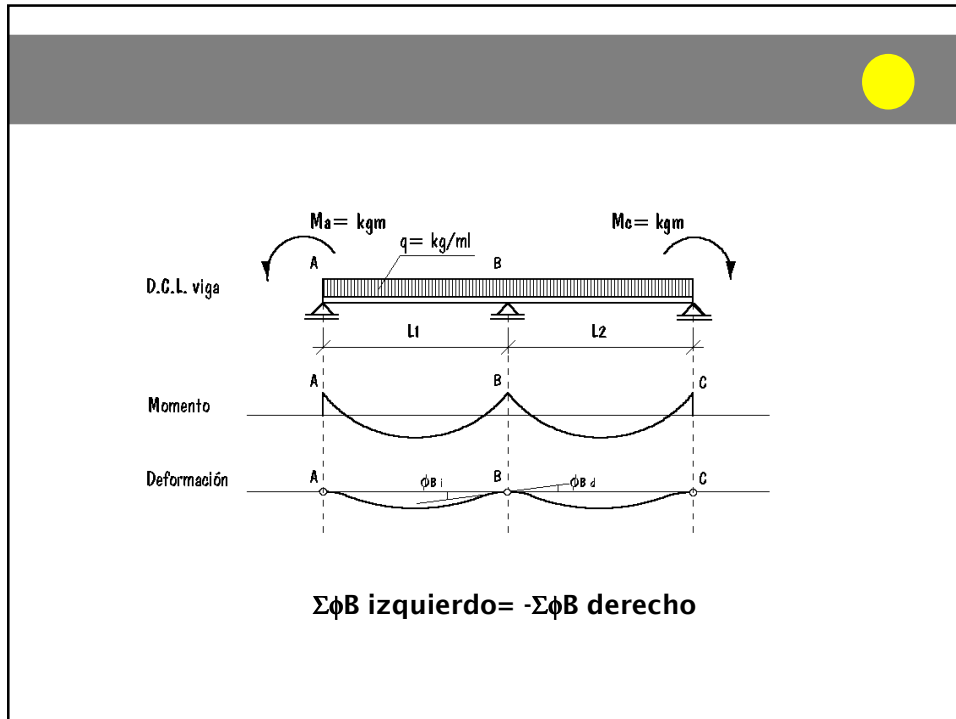
$$M_{MAX} = \frac{9qL^2}{128}$$





Teorema de los tres momentos o Clapeyron





$$\Sigma\phi_B \text{ izquierdo} = -\Sigma\phi_B \text{ derecho}$$

$$-\frac{qL_1^3}{24EI} + \frac{MaL_1}{6EI} + \frac{MbL_1}{3EI} = -\left[-\frac{qL_2^3}{24EI} + \frac{MbL_2}{3EI} + \frac{McL_2}{6EI} \right]$$

$$\frac{MaL_1}{6EI} + \frac{MbL_1}{3EI} + \frac{MbL_2}{3EI} + \frac{McL_2}{6EI} = \frac{qL_1^3}{24EI} + \frac{qL_2^3}{24EI}$$

Reemplazando L/EI por λ (módulo de flexibilidad)

$$\frac{Ma\lambda_1}{6} + \frac{Mb\lambda_1}{3} + \frac{Mb\lambda_2}{3} + \frac{Mc\lambda_2}{6} = \frac{qL_1^2\lambda_1}{24} + \frac{qL_2^2\lambda_2}{24} \quad /*6$$

Al amplificar la expresión 6 veces se obtiene

$$Ma\lambda_1 + 2Mb\lambda_1 + 2Mb\lambda_2 + Mc\lambda_2 = 6 \left[\frac{qL_1^2\lambda_1}{24} + \frac{qL_2^2\lambda_2}{24} \right]$$

$$Ma\lambda_1 + 2Mb(\lambda_1 + \lambda_2) + Mc\lambda_2 = 6 \left[\frac{qL_1^2\lambda_1}{24} + \frac{qL_2^2\lambda_2}{24} \right]$$

Si $EI = \text{constante}$ y $\lambda = L/EI$ $\lambda = L$

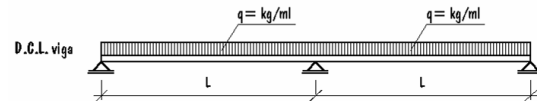
Reemplazando $\lambda = L$ en la ecuación se tiene:

$$MaL_1 + 2Mb(L_1 + L_2) + McL_2 = 6 \left[\frac{qL_1^3}{24} + \frac{qL_2^3}{24} \right]$$

Reemplazando $\frac{qL_1^3}{24}$ por Tc_1 y $\frac{qL_2^3}{24}$ por Tc_2

$$\boxed{MaL_1 + 2Mb(L_1 + L_2) + McL_2 = 6(Tc_1 + Tc_2)}$$

Ejemplo Viga de dos tramos con carga uniformemente repartida



$$MaL_1 + 2Mb(L_1+L_2) + McL_2 = 6(Tc_1 + Tc_2)$$

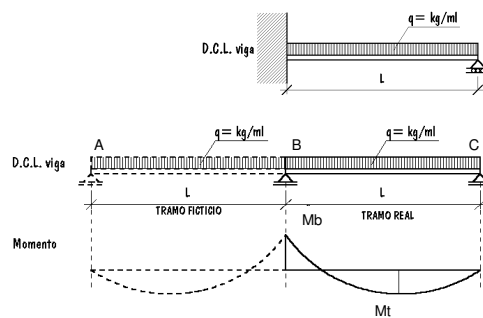
$$0L_1 + 2Mb(L_1+L_2) + 0L_2 = 6 \left[\frac{qL_1^3}{24} + \frac{qL_2^3}{24} \right]$$

$$2Mb(L_1+L_2) = \frac{qL_1^3}{4} + \frac{qL_2^3}{4} \quad \text{Si } L_1=L_2$$

$$2Mb \cdot 2L = \frac{qL^3}{2}$$

$$Mb = \frac{qL^2}{8}$$

Ejemplo Viga empotrada en un extremo y apoyada en el otro con carga uniformemente repartida



$$MaL_1 + 2Mb(L_1+L_2) + McL_2 = 6(Tc_1 + Tc_2)$$

$$00 + 2Mb(0+L) + 0L = 6 \left[0 + \frac{qL^3}{24} \right]$$

$$Mb = \frac{qL^2}{8}$$

$$2Mb L = \frac{qL^3}{4}$$